GENERAL NOTES

G1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS. ALL DISCREPANCIES SHALL BE REFERRED TO THE ENGINEER FOR DECISION BEFORE PROCEEDING WITH THE WORK.

G2. ALL DIMENSIONS RELATIVE TO SETTING OUT AND OFF SITE WORK SHALL BE VERIFIED BY THE CONTRACTOR BEFORE CONSTRUCTION AND FABRICATION IS COMMENCED. THESE DRAWINGS SHALL NOT BE SCALED.

G3. DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE IN A SAFE AND STABLE CONDITION AND SHALL ENSURE THAT NO PART IS OVERSTRESSED DUE TO CONSTRUCTION ACTIVITIES.

G4. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE RELEVANT SAA CODES AND LOCAL STATUTORY AUTHORITIES.

G5. NO SUBSTITUTIONS SHALL BE MADE OR SIZES OF STRUCTURAL MEMBERS VARIED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER. THE APPROVAL OF A SUBSTITUTION FROM THE ENGINEER SHALL NOT BE AN AUTHORISATION FOR AN EXTRA. ANY EXTRA INVOLVED SHALL BE APPROVED WITH THE ENGINEER BEFORE WORK COMMENCES.

G6. ALL DIMENSIONS ARE IN MILLIMETRES UNLESS STATED OTHERWISE. ALL LEVELS ARE EXPRESSED IN METRES, AND ARE TO THE STRUCTURAL SURFACE UNLESS NOTED OTHERWISE.

SAFETY–IN–DESIGN REVIEW

MAGRYN & ASSOCIATES (MAGRYN) HAVE CONDUCTED A PRELIMINARY SAFETY–IN–DESIGN REVIEW OF THE DESIGN SHOWN ON THESE DRAWINGS. THE REVIEW IS BASED GENERALLY ON THE PROCEDURE OUTLINED IN THE SAFE WORK AUSTRALIA PUBLICATION "SAFE DESIGN OF STRUCTURES CODE OF PRACTICE" (JULY 2012).

THE DESIGN HAS NOT BEEN REVIEWED WITH A CONTRACTOR/BUILDER AT THE TIME OF ISSUE FOR TENDER OR CONSTRUCTION. CONSTRUCTION METHODS VARY BETWEEN CONTRACTORS SO IT IS NOT POSSIBLE FOR MAGRYN TO PERFORM AN EXHAUSTIVE SAFETY–IN–DESIGN OR SAFETY–IN–CONSTRUCTION REVIEW. ONCE APPOINTED, THE CONTRACTOR IS REQUIRED TO UNDERTAKE A THOROUGH REVIEW OF THE DESIGN WITH THEIR SUB–CONTRACTORS TO IDENTIFY SAFETY RISKS DURING CONSTRUCTION AND DURING THE LIFE OF THE BUILDING.

CONTRACTORS ARE RESPONSIBLE TO REVIEW THEIR PROPOSED ERECTION PROGRAMS/SEQUENCE AND FOR THE TEMPORARY FRAMING TO SUPPORT STRUCTURAL ELEMENTS.

CONTRACTORS SHALL PROVIDE DOCUMENTATION THAT OUTLINE HOW THE PROJECT WAS BUILT SO THAT THE DEMOLITION CONTRACTOR CAN ADEQUATELY EVALUATE RISKS DURING DEMOLITION PLANNING AT THE END OF THE LIFE OF THE BUILDING.

LOCATION OF UNDERGROUND AND ABOVE GROUND SERVICES

1. MAGRYN HAS NOT CARRIED OUT A DIAL–BEFORE–YOU–DIG REVIEW DURING THE DESIGN PHASE. THE CONTRACTOR SHALL UNDERTAKE A REVIEW OF THEIR OWN TO VERIFY SERVICES ENTERING THE PROPERTY AND IN PROXIMITY TO THE BOUNDARY IN THE STREET/SURROUNDING THE PROPERTY.
2. DIAL–BEFORE–YOU–DIG DOES NOT CONFIRM THE LAYOUT OF SERVICES WITHIN THE SITE. THE CONTRACTOR SHALL ALLOW TO ENGAGE A SERVICES LOCATION CONTRACTOR TO CONDUCT A SURVEY OF ALL SERVICES ON THE SITE.

VERIFICATION OF SOIL CONDITIONS

1. THE GEOTECHNICAL INVESTIGATION WAS BASED ON A LIMITED SURVEY VIA BORE HOLES IN DISCRETE LOCATIONS AROUND THE SITE (REFER TO THE GEOTECHNICAL REPORT REFERENCED ON THESE DRAWINGS). THE CONTRACTOR SHALL HAVE THE SOIL DESIGN PARAMETERS VERIFIED DURING EXCAVATION. ALLOW TO ENGAGE THE GEOTECHNICAL ENGINEER TO CONDUCT A REVIEW OF THE SOIL DURING CLEARING / EXCAVATION OF THE SITE.

CONCRETE

C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600. ALL CONCRETE SHALL BE TESTED BY AN APPROVED NATA INDEPENDENT TESTING LABORATORY.

C2. CONCRETE SHALL BE GRADE N32.

C3. COVER TO STEEL REINFORCEMENT SHALL BE 50mm UNLESS SHOWN OTHERWISE ON THE DRAWINGS. EXPOSURE CLASSIFICATION TO AS3600 IS A1.

C4. CONDUITS SHALL NOT BE PLACED WITHIN THE CONCRETE COVER.

C5. CONCRETE ADDITIVES SHALL NOT BE USED WITHOUT THE APPROVAL OF THE ENGINEER.

C6. FINISHES TO ALL CONCRETE SURFACES SHALL BE STEEL FLOAT.

C7. SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES AND SHALL NOT BE ALTERED WITHOUT THE APPROVAL OF THE ENGINEER.

C8. FREE DROPPING OF CONCRETE FROM A HEIGHT GREATER THAN 1200mm SHALL NOT BE PERMITTED.

C9. CONCRETE SHALL BE COMPACTED WITH SUITABLE MECHANICAL VIBRATORS TO AS3600.

C10. CONCRETE SHALL BE PLACED IN ONE CONTINUOUS POURING OPERATION.

C11. REINFORCEMENT SHALL BE SUPPORTED ON APPROVED PLASTIC STOOLS OR MORTAR BLOCKS OF EQUAL STRENGTH AND DURABILITY TO THE CONCRETE MIX, AT NOT MORE THAN 800mm CENTRES.

C12. CURE CONCRETE (WATER TO BE POTABLE) FOR 7 DAYS AFTER FINAL POUR. WATER CURE WITH PLASTIC MEMBRANE, FLOODED.

C13. CONCRETE SHALL NOT BE PLACED IN THE WORKS IF THE TEMPERATURE OF THE SURROUNDING AIR FALLS BELOW 5 DEGREES(°) CELSIUS(C), OR IS HIGHER THAN 32°C OR WIND SPEEDS EXCEED 25km/h.

C14. REINFORCING SYMBOLS:
N – GRADE 500, HOT ROLLED DEFORMED BAR, COMPLYING WITH AS4671.

C15. THE CONTRACTOR SHALL ARRANGE FOR THE ENGINEER TO INSPECT THE REINFORCEMENT AND OBTAIN HIS APPROVAL PRIOR TO POURING CONCRETE.

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PROJECT:
ADDITIONS TO RESIDENCE

PROJECT ADDRESS:
31 ZEPHYR TERRACE,
PORT WILLUNGA

TITLE:
NOTES 1/2

CONTRACTORS MUST VERIFY ALL DIMENSIONS PRIOR TO ANY OFF SITE FABRICATION.

DESIGN: BJ	SCALE: AS SHOWN	DATE: JUL. 2022
SHEET SIZE: A3	DRAWING NUMBER: 22201–1	REVISION: A

STEELWORK NOTES

S1. CONSTRUCTION USING THESE STRUCTURAL DRAWINGS SHALL ONLY COMMENCE IF THE STRUCTURAL STEEL DRAWINGS ARE DESIGNATED “ISSUED FOR CONSTRUCTION”.

S2. DIMENSIONS SHALL BE TAKEN FROM ARCHITECTURAL DRAWINGS ONLY. STRUCTURAL STEEL DRAWINGS SHALL NOT BE SCALED TO OBTAIN DIMENSIONS.

S3. THE STRUCTURAL STEEL SHOWN ON THESE DRAWINGS HAS BEEN DESIGNED IN ACCORDANCE WITH AS 4100.

S4. NO CHANGES OR SUBSTITUTIONS MAY BE MADE TO ANY STRUCTURAL STEEL ELEMENT DOCUMENTED IN THESE STRUCTURAL STEEL DRAWINGS WITHOUT REFERENCE TO AND APPROVAL BY THE STRUCTURAL ENGINEER.

S5. THESE STRUCTURAL STEEL DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER RELEVANT DRAWINGS. ANY DISCREPANCIES IN THE STRUCTURAL STEEL DRAWINGS SHALL BE REFERRED TO THE STRUCTURAL ENGINEER FOR RESOLUTION.

S6. FABRICATION AND ERECTION OF THE STRUCTURAL STEEL SHALL COMPLY WITH SECTIONS 14 AND 15 RESPECTIVELY OF AS 4100.

S7. DURING ERECTION, THE STRUCTURAL STEEL SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERLOADED. TEMPORARY BRACING NOT SHOWN ON THE STRUCTURAL STEEL DRAWINGS SHALL BE PROVIDED BY THE BUILDER AS REQUIRED.

S8. UNLESS NOTED OTHERWISE IN THE MEMBER SCHEDULE, ALL STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING AUSTRALIAN STANDARDS IN RESPECT OF GRADE AND CONDITIONS OF SUPPLY:

ROLLED SECTIONS AS/NZS 3679.1 GRADE 300
WELDED SECTIONS AS/NZS 3679.2 GRADE 300
HOLLOW SECTIONS AS/NZS 1163 GRADE C350

S9. ALL BEAMS AND RAFTERS SHALL BE SUPPLIED WITH ANY NATURAL CAMBER UP.

S10. ALL EXTERNAL/EXPOSED STEELWORK FURTHER THAN 100m FROM THE COAST TO BE HOT DIPPED GALVANISED TO AS 4680–1999–HDG 390g/m2 (MIN.), UNLESS NOTED OTHERWISE OR ALTERNATIVELY APPROVED. ALL INTERNAL STEELWORK TO BE HOT DIPPED GALVANISED TO AS 4680–1999–HDG 390g/m2 (MIN.), UNLESS NOTED OTHERWISE OR ALTERNATIVELY APPROVED.

S11. BOLTING CATEGORIES SHOWN ON THESE STRUCTURAL STEEL DETAILS SHALL BE THOSE DEFINED IN CLAUSE 9.3.1 OF AS 4100 (NAMELY 4.6/S, 8.8/S, 8.8/TB, 8.8/TF). TEST CERTIFICATES CONFIRMING FULL COMPLIANCE WITH THE RELEVANT AUSTRALIAN STANDARDS (AS 1111 OR AS/NZS 1252) AND ANY OTHER AUSTRALIAN STANDARDS CITED THEREIN SHALL BE SUPPLIED TO THE STRUCTURAL ENGINEER.

S12. BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH CLAUSE 15.2.3 OF AS 4100 AND BOLTS REQUIRING TENSIONING (8.8/TB AND 8.8/TF) SHALL BE INSTALLED IN ACCORDANCE WITH CLAUSES 15.2.4 AND 15.2.5 OF AS 4100 USING EITHER THE PART–TURN METHOD OR A DIRECT–TENSION INDICATION DEVICE. THE TORQUE CONTROL METHOD SHALL NOT BE USED.

S13. 8.8 GRADE BOLTS SHALL NOT BE WELDED OR BENT.

S14. UNLESS NOTED OTHERWISE ALL BOLTED CONNECTIONS SHALL BE 2M20 8.8/S WITH 10mm THICK CLEATS.

S15. UNLESS NOTED OTHERWISE, ALL BOLTS, NUTS AND WASHERS SHALL BE GALVANISED.

S16. ALL BOLT HOLES SHALL BE 2mm LARGER THAN THE NOMINAL BOLT DIAMETER EXCEPT WHERE SLOTTED OR OVERSIZE HOLES ARE SHOWN ON THE STRUCTURAL STEEL DETAILS. ALL HOLES SHALL COMPLY WITH CLAUSE 14.3.5 OF AS 4100. PLATE WASHERS SHALL BE PROVIDED WHERE REQUIRED BY CLAUSE 14.3.5.

S17. ALL CUT SURFACES SHALL COMPLY WITH CLAUSE 14.3.3 OF AS 4100.

S18. ALL WELDING SHALL COMPLY WITH AS/NZS 1554.1 UNLESS NOTED OTHERWISE ON THE STRUCTURAL STEEL DETAILS. WELDING CATEGORY SHALL BE SP UNLESS NOTED OTHERWISE. ALL WELD METAL SHALL HAVE A NOMINAL WELD METAL TENSILE STRENGTH OF 430 MPA UNLESS NOTED OTHERWISE.

S19. UNLESS OTHERWISE NOTED WELDS SHALL BE 6mm CONTINUOUS FILLET (CFW).

S20. ALL WELDING PERSONNEL SHALL BE QUALIFIED IN ACCORDANCE WITH CLAUSE 4.12 OF

STEELWORK NOTES CONT.

S21. FOR BASE PLATES, PROVIDE A TEMPLATE WITH SETTING OUT LINES CLEARLY MARKED OR CAGE ANCHOR BOLT GROUP IN ORDER TO ENSURE ACCURATE POSITIONING OF ANCHOR BOLTS TO BE CAST IN. SUPPORT STEEL ATTACHED TO BASE PLATE WITH STEEL WEDGES /PACKERS UNTIL GROUTED.

S22. CONNECTION CLEATS SHOWN ON THE STRUCTURAL DRAWINGS WHERE NOT IDENTIFIED SPECIFICALLY, SHALL BE EITHER GRADE 250 PLATE TO AS/NZS 3678 OR GRADE 300 FLAT BAR TO AS/NZS 3679.1.

S23. PROPRIETARY CLEATS SHALL BE PROVIDED BY THE FABRICATOR FOR THE CONNECTION OF COLD FORMED SECTIONS. USE 8mm THICK CLEATS AND 2M12 4.6/S BOLTS UNLESS NOTED OTHERWISE.

S24. BASE PLATE GROUT FOR STEELWORK SHALL BE AN APPROVED NON SHRINK CEMENTITIOUS GROUT WITH A 28 DAY STRENGTH OF AT LEAST 30MPA AND SHALL BE OF A CONSISTENCY SUCH THAT IT CAN PROVIDE UNIFORM BEARING UNDER THE ENTIRE SURFACE OF THE STEELWORK.

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31 ZEPHYR TERRACE, PORT WILLUNGA

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CONTRACTORS MUST VERIFY ALL DIMENSIONS PRIOR TO ANY OFF SITE FABRICATION.

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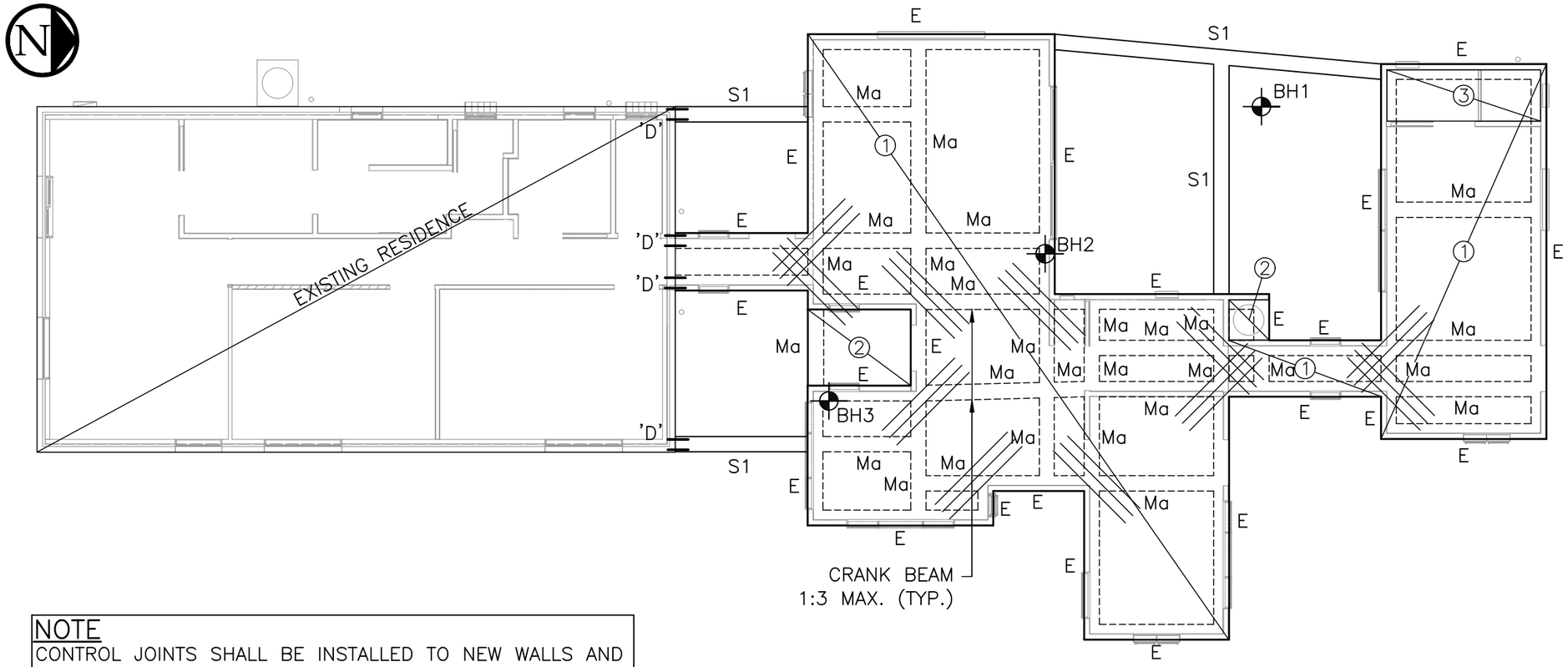

FOOTING PLAN

SCALE 1:100

NOTE
CONTROL JOINTS SHALL BE INSTALLED TO NEW WALLS AND BETWEEN NEW/EXISTING WALLS IN ACCORDANCE WITH THE CLADDING MANUFACTURER'S SPECIFICATIONS.

LEGEND

- ⊕ – APPROXIMATE BOREHOLE LOCATIONS.
- 'D' – 4 N12 DOWELS – REFER DWG. 22201–7.
- /// – RE–ENTRANT CORNER BARS, 3N12, 2000mm LONG, 200mm APART, FIX TO UNDERSIDE OF SLAB REINFORCEMENT.
- (1) – 100mm THICK SLAB INTEGRAL WITH FOOTING BEAMS, SL82 MESH TOP, 30mm COVER, N25 CONCRETE.
- (2) – EXTERNAL SETDOWNS AND FALLS TO ARCHITECT'S DETAIL.
- (3) – WET AREA SETDOWN AND FALLS TO ARCHITECT'S DETAIL.

FOOTING BEAM SCHEDULE

BEAM	DEPTH	WIDTH	TOP STEEL	BOTTOM STEEL	LIGATURES O.N.O.
E	600	300	2N16	3N16	W8–900CTS
Ma	600	300	2N16	3N16	W8–900CTS
S1	600	300	2N16	3N16	W8–900CTS

FOOTING NOTE

1. REFER TO CONSTRUCTION REPORT FOR ADDITIONAL DETAILS AND DRAWINGS REGARDING THE CONCRETE RAFT SLAB.
3. USE 2 LAYERS SL82 MESH (TOP) IN SLAB IN AREAS TO BE TILED. STAGGER MESH SUCH THAT REINFORCING WIRES OCCUR @ 100 CTS. EACH WAY.
4. PROVIDE 2 LAYERS OF DAMP PROOF MEMBRANE IN FOOTING BEAMS DEEPER THAN 1100mm.
5. WHERE FILL ONSITE EXCEEDS 400mm THE SLAB SHALL BE THICKENED TO 125mm AND SL82 MESH SHALL BE PLACED TOP AND BOTTOM. TOP MESH SHALL HAVE A CLEAR CONCRETE COVER OF 30mm AND BOTTOM MESH SHALL HAVE A CLEAR CONCRETE COVER OF 40mm. LIGATURE SPACING TO BE REDUCED 300mm CTS.
6. USE N25 CONCRETE.
7. SITE CLASSIFICATION BASED ON SOIL REACTIVITY TO AS2870–2011 IS H1–D, P (FILL).
8. ALL FOOTINGS TO BE CONTINUOUSLY TRENCHED OR PIERED MIN. 200mm INTO FIRM NATURAL GROUND.
9. S1 DENOTES STRIP FOOTING. SET DOWN BELOW BASE OF PAVING.

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FOOTING PLAN

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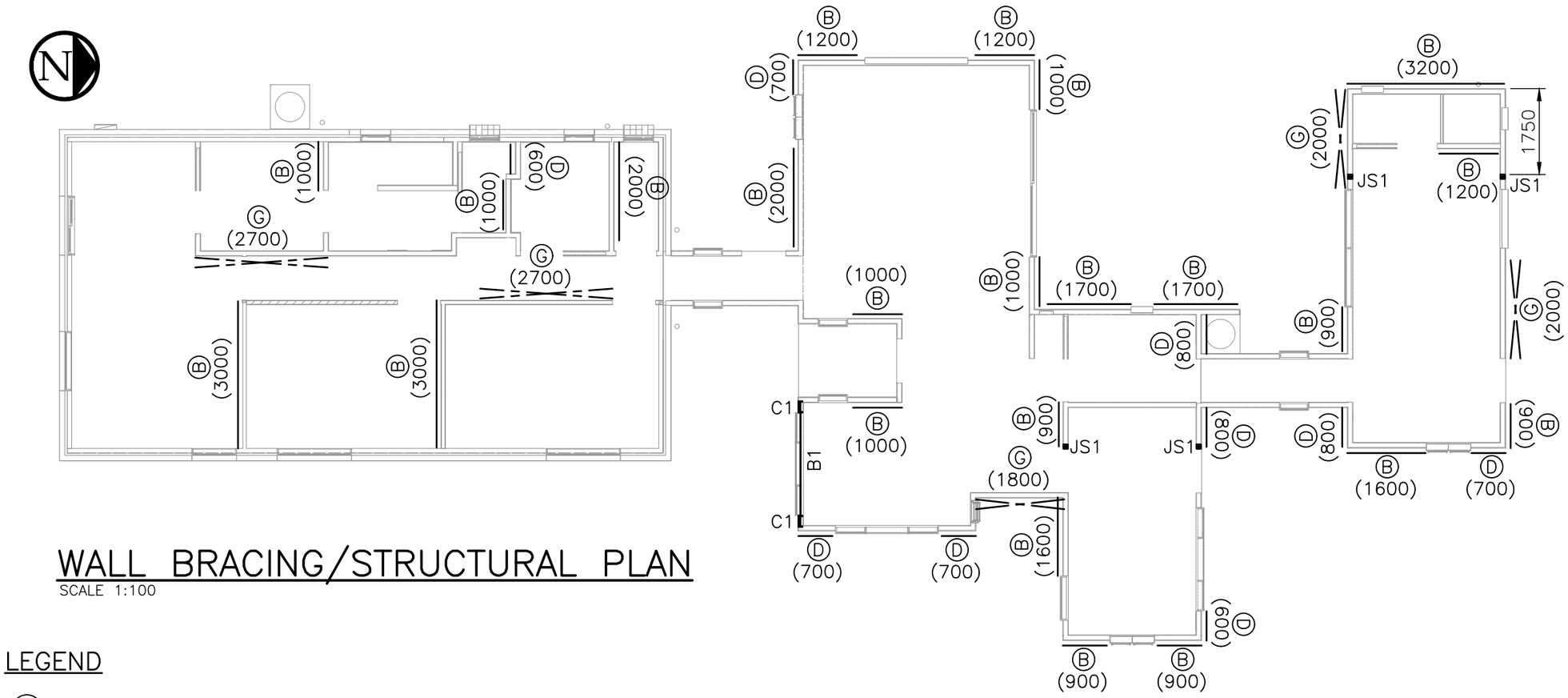

WALL BRACING/STRUCTURAL PLAN

SCALE 1:100

LEGEND

Ⓑ – HARDBOARD, TYPE B, SHALL COMPLY WITH AS/NZS 1859.4.
HARDBOARD SHALL BE NAILED TO FRAME USING MINIMUM 30x2.8mmø GALVANIZED FLAT–HEAD NAILS OR EQUIVALENT.
NAILS SHALL BE LOCATED A MINIMUM OF 10mm FROM THE VERTICAL EDGES AND 15mm FROM THE TOP AND BOTTOM EDGES.
MAXIMUM STUD SPACING = 600mm.
BRACING PANEL MINIMUM WIDTH = 900mm.
PANEL EDGES SHALL BE SUPPORTED BY STUDS.
MINIMUM HARDBOARD THICKNESS: 4.8mm
FASTENER SPACING: TOP AND BOTTOM PLATES 40mm
VERTICAL EDGES AND NOGGING 150mm
INTERMEDIATE STUDS 300mm
FIXING OF BOTTOM PLATE TO FLOOR FRAME OR SLAB WITH M10 BOLTS EACH END AND INTERMEDIATELY AT MAX. 1200mm CENTRES.

Ⓓ – HARDBOARD, TYPE D, SHALL COMPLY WITH AS/NZS 1859.4.
HARDBOARD SHALL BE NAILED TO FRAME USING MINIMUM 30x2.8mmø GALVANIZED FLAT–HEAD NAILS OR EQUIVALENT.
NAILS SHALL BE LOCATED A MINIMUM OF 10mm FROM THE VERTICAL EDGES AND 15mm FROM THE TOP AND BOTTOM EDGES.
MAXIMUM STUD SPACING = 600mm.
BRACING PANEL MINIMUM WIDTH = 460mm.
PANEL EDGES SHALL BE SUPPORTED BY STUDS.
MINIMUM HARDBOARD THICKNESS: 4.8mm
FASTENER SPACING: TOP AND BOTTOM PLATES 80mm
VERTICAL EDGES AND NOGGING 150mm
FIXING OF BOTTOM PLATE TO FLOOR FRAME OR SLAB WITH NOMINAL FIXING REQUIREMENT.
INSTALL M10x50mm LONG COACH SCREW WITH 30x38mm WASHER AT EACH CORNER OF PANEL.

Ⓖ – DOUBLE DIAGONAL TENSION OR METAL STRAP BRACES WITH STUD STRAPS 30x0.8mm TENSIONED METAL STRAP FIXED TO STUDS WITH 1/30x2.8mmø GALVANIZED FLAT–HEAD NAIL (OR EQUIVALENT) AND TO PLATES WITH 4/30x2.8mmø GALVANIZED FLAT–HEAD NAILS, OR ALTERNATIVE METAL STRAP, FIXED AS ABOVE, WITH A NET SECTIONAL AREA NOT LESS THAN 21mm².
30x0.8mm GALVANIZED METAL STRAP LOOPED OVER PLATE AND FIXED TO STUD WITH 4/30x2.8mmø GALVANIZED FLAT–HEAD NAILS (OR EQUIVALENT) TO EACH END. ALTERNATIVELY, PROVIDE SINGLE STRAPS TO BOTH SIDES, WITH 4 NAILS PER STRAP END, OR EQUIVALENT ANCHORS OR OTHER FASTENERS.
FIX BOTTOM PLATE TO FLOOR FRAME OR SLAB WITH NOMINAL FIXING REQUIREMENT.

MEMBER SCHEDULE

MARK	SIZE	COMMENTS
C1	230PFC, GRADE 300	PORTAL FRAME COLUMN, 12mm THICK BASEPLATE 2M16 CHEMSET ANCHORS. REFER B1 TO C1 TYPICAL DETAIL ON DWG. 22201–5.
B1	230PFC, GRADE 300	PORTAL FRAME BEAM, MAX. 2100mm SPAN, 2M16 8.8/S BOLTS TO 10mm CLEAT ON C1. REFER B1 TO C1 TYPICAL DETAIL ON DWG. 22201–5.
JS1	2/90x45 MGP10	TIMBER JAMB STUDS. NAIL LAMINATE IN ACCORDANCE WITH AS 1684.2.

NOTE: B1 TO BE AT FULL HEIGHT TO SUPPORT RAFTERS. SEPARATE WINDOW LINTEL SHALL BE CONNECTED TO C1 VIA 10mm CLEAT, 3M12 4.6/S BOLTS.

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TITLE:
WALL FRAMING/BRACING PLAN

CONTRACTORS MUST VERIFY ALL DIMENSIONS PRIOR TO ANY OFF SITE FABRICATION.

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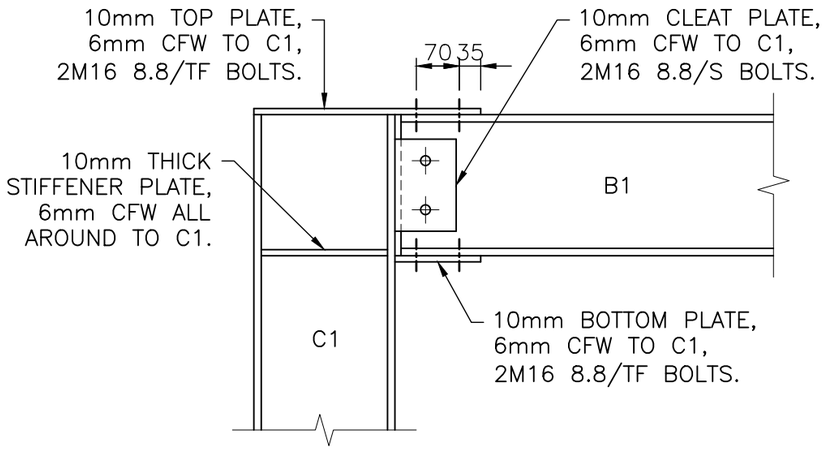
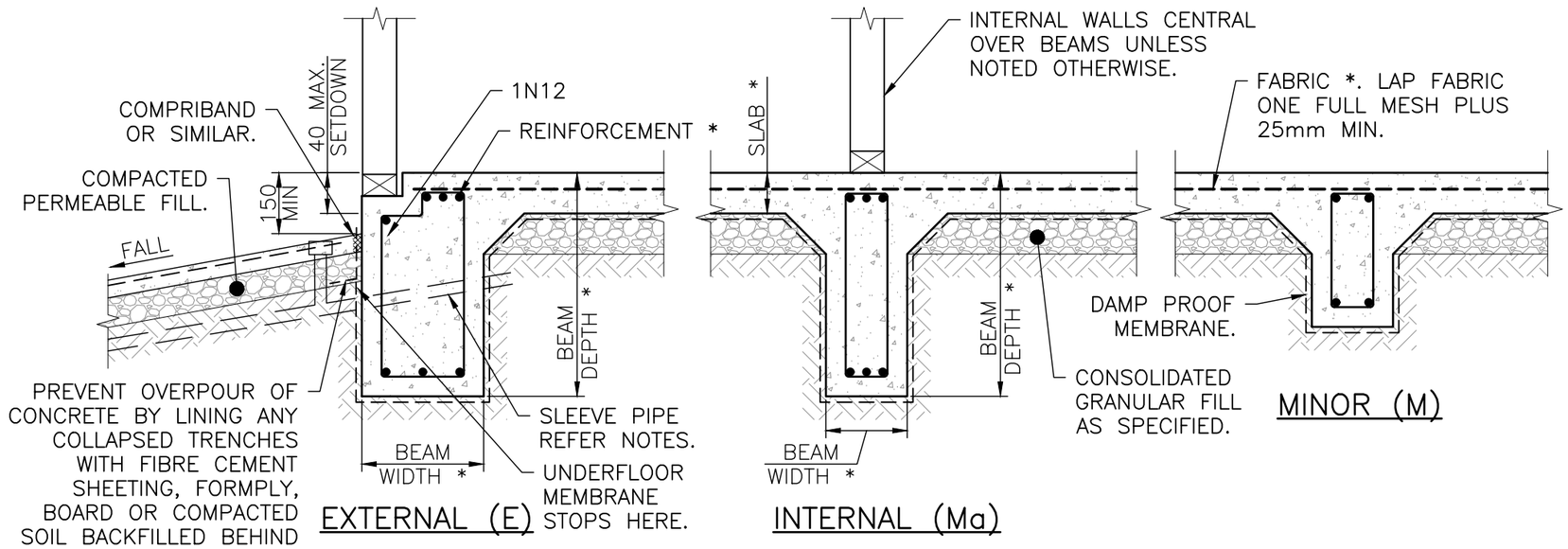


TYPICAL B1 TO C1 DETAIL

SCALE 1:10

TIE DOWNS FOR EXISTING AND NEW

BATTENS TO RAFTERS:	1/75 No.14 TYPE 17 SCREWS @ 600mm MAX. CTS.
RAFTERS/TRUSSES TO WALL FRAME:	PROVIDE 1 NO. 30x0.8 GI STRAP WITH 3x2.8ø NAILS EACH END OF STRAP @ 600mm MAX. CTS.
STUDS TO BOTTOM PLATES:	30x0.8 GI STRAPS, 2x2.8ø NAILS PER END @ 600mm MAX. CTS.
BOTTOM PLATE TO SLAB:	1xM10 DYNABOLT @ 600mm MAX. CTS. 100mm MAX. FROM EACH SIDE OF JS1.
JS1 TO ROOF BEAM:	30x0.8 GI STRAPS, 4x2.8ø NAILS PER END.
JS1 TO BOTTOM PLATE:	30x0.8 GI STRAPS, 4x2.8ø NAILS PER END.



STIFFENED RAFT TYPICAL BEAM CROSS SECTIONS

SCALE N.T.S

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TIE DOWNS, SECTIONS & DETAIL

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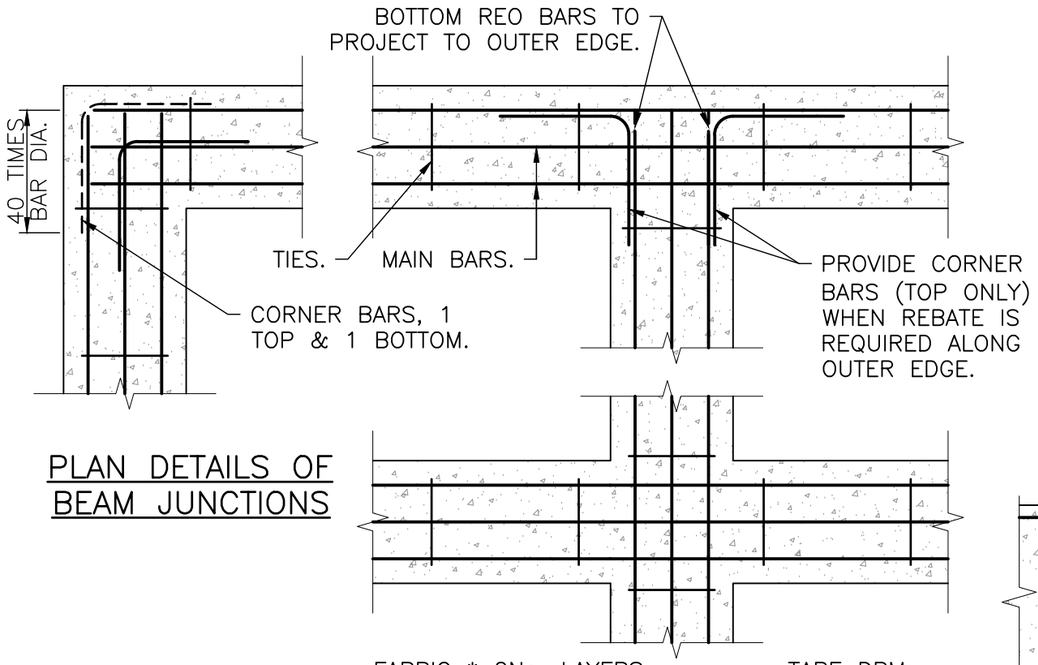
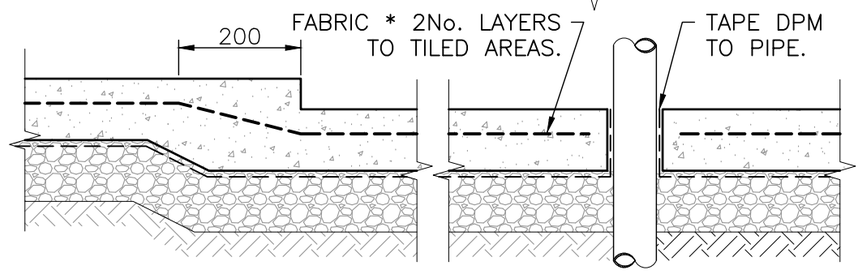


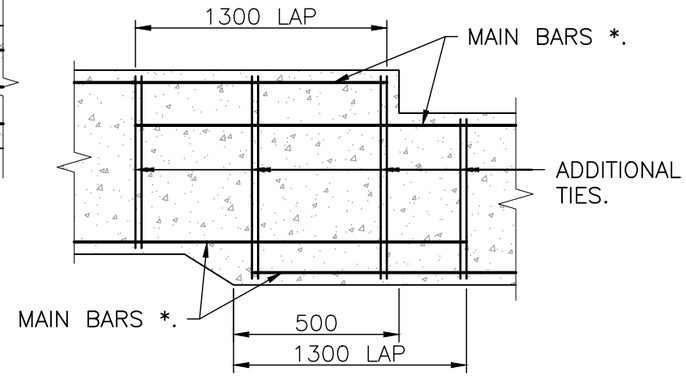
PLAN DETAILS OF BEAM JUNCTIONS



SLAB SECTION

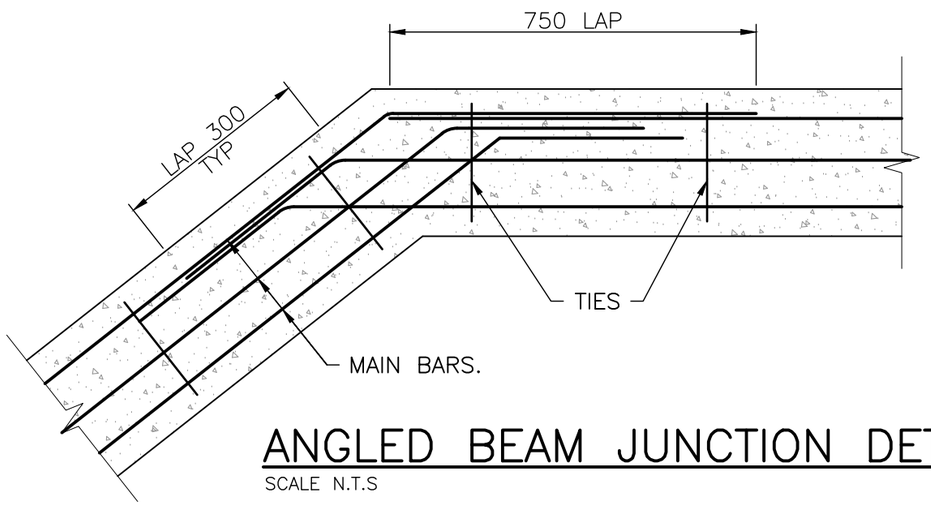
NOTES

1. MAIN BARS TO BE CONTINUOUS WHERE POSSIBLE SPLICES WHEN NECESSARY AS SPECIFIED *.
2. * – REFER TO CONSTRUCTION REPORT.
3. CLOSED CELL POLYETHYLENE LAGGING SHALL BE USED AROUND ALL STORMWATER AND SEWER PIPE PENETRATIONS THROUGH FOOTINGS THE LAGGING SHALL BE 40mm ON CLASS E & H SITES AND 20mm ON CLASS S & M SITES. SLEEVES ALLOWING EQUIVALENT MOVEMENT MAY BE USED AS AN ALTERNATIVE.



STIFFENED RAFT GENERAL DETAILS

SCALE N.T.S



ANGLED BEAM JUNCTION DETAILS

SCALE N.T.S

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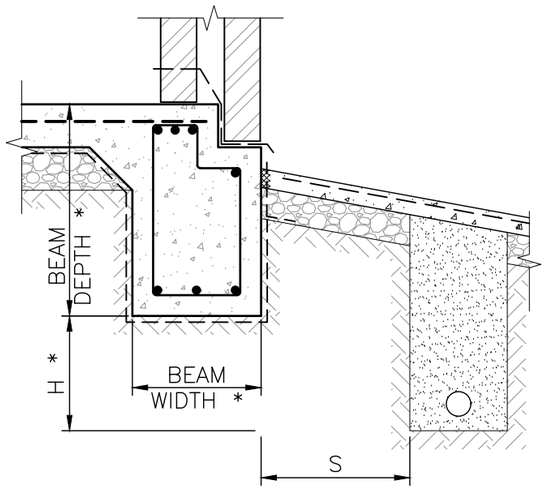
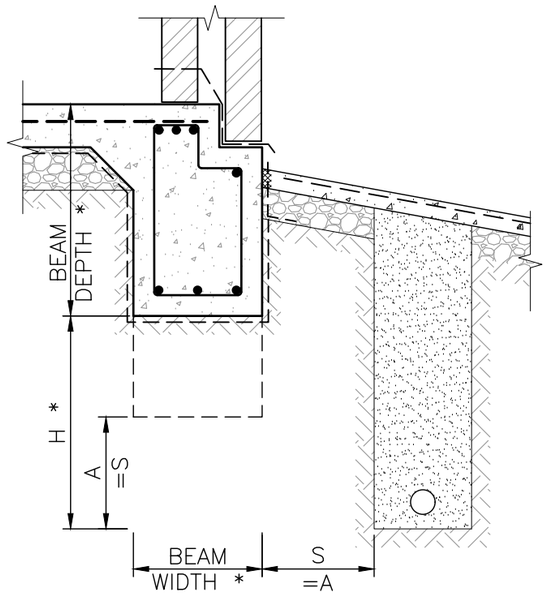
DESIGN: BJ	SCALE: AS SHOWN	DATE: JUL. 2022
SHEET SIZE: A3	DRAWING NUMBER: 22201–6	REVISION: A



SITUATION A

WHEN DEPTH 'H' IS LESS THAN DISTANCE 'S' THE FOOTING DEPTH SPECIFIED REMAINS UNALTERED.



SITUATION B

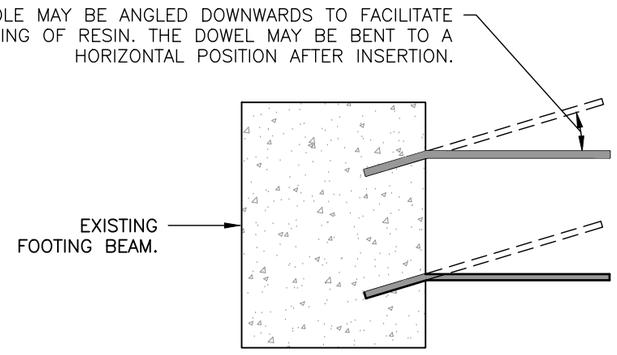
WHEN DEPTH 'H' IS GREATER THAN DISTANCE 'S' PROVIDE MINIMUM 1.0m LONG MASS CONCRETE PIERS THE SAME WIDTH AS THE FOOTING BEAM AT MAXIMUM 3.0m CENTRES TO THE DEPTH SHOWN. ALTERNATIVELY THE BEAM DEPTH MAY BE INCREASED TO ACHIEVE THE NECESSARY FOUNDING DEPTH.

NOTES

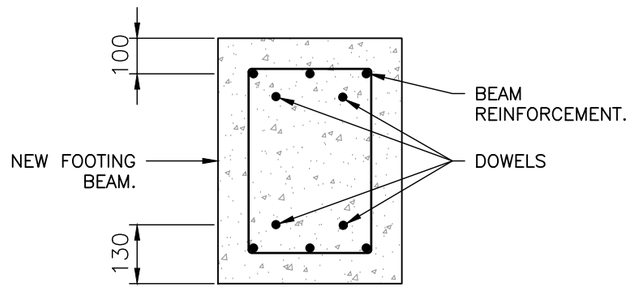
1. * – REFER TO CONSTRUCTION REPORT.
2. BACKFILL TRENCH WITH A SANDY MATERIAL IN LAYERS NOT EXCEEDING 200mm THICK. EACH LAYER SHALL BE COMPACTED BY APPROVED METHODS TO THE REQUIRED DENSITY BEFORE THE NEXT LAYER IS PLACED. THE BACKFILL SHALL BE FREE OF CLAY LUMPS AND BUILDING DEBRIS.

STIFFENED RAFT REQUIREMENTS ADJACENT PIPE TRENCHES

SCALE N.T.S



SECTION THROUGH EXISTING FOOTING



SECTION THROUGH NEW FOOTING

AT LOCATIONS MARKED 'D' ON FOOTING PLAN

IF PRACTICABLE, DOWEL NEW FOOTINGS TO EXISTING FOOTINGS WITH 4No. N12 x 500 LONG DOWELS BONDED INTO HOLES DRILLED 150mm INTO EXISTING FOOTINGS. USE MASTER BUILDERS 'EPOXANWELD FMF' OR EQUAL APPROVED.

NOTE: ALL DIMENSIONS IN MILLIMETRES.

DOWELLING OF FOOTINGS NEW TO EXISTING

SCALE N.T.S

ISSUE	AMENDMENTS	INT./DATE
A	PRELIMINARY ISSUE	TH 05.08.22

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PROJECT:
ADDITIONS TO RESIDENCE

PROJECT ADDRESS:
31 ZEPHYR TERRACE, PORT WILLUNGA

TITLE:
DETAILS

CONTRACTORS MUST VERIFY ALL DIMENSIONS PRIOR TO ANY OFF SITE FABRICATION.

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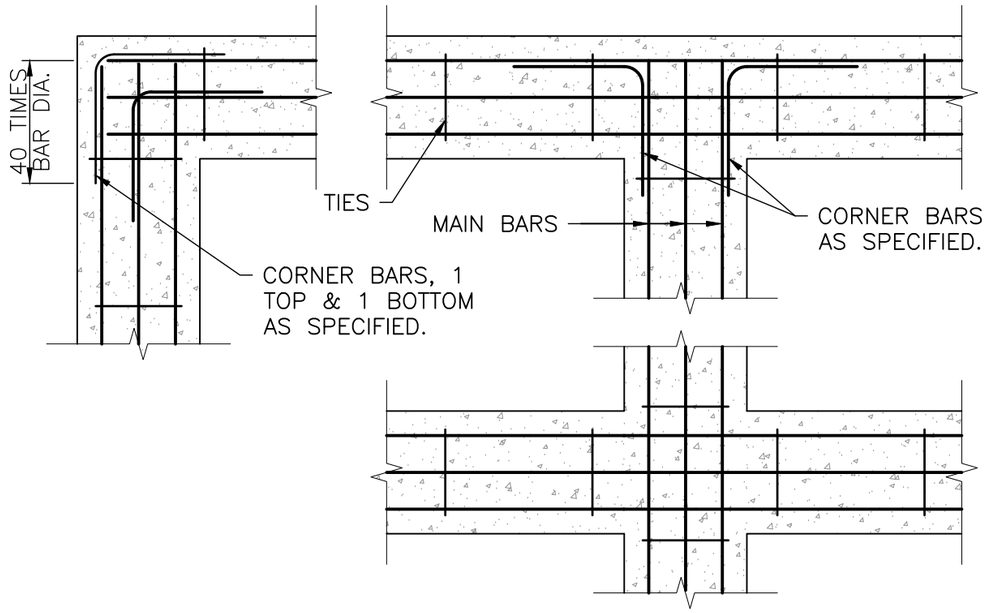
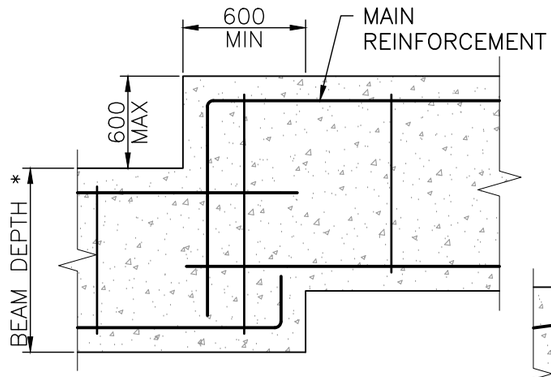
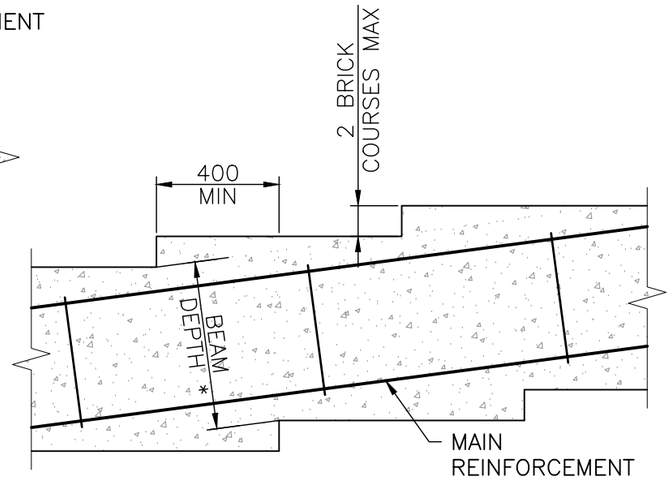


PLAN DETAILS AT STRIP FOOTING JUNCTIONS



FOOTING ELEVATION



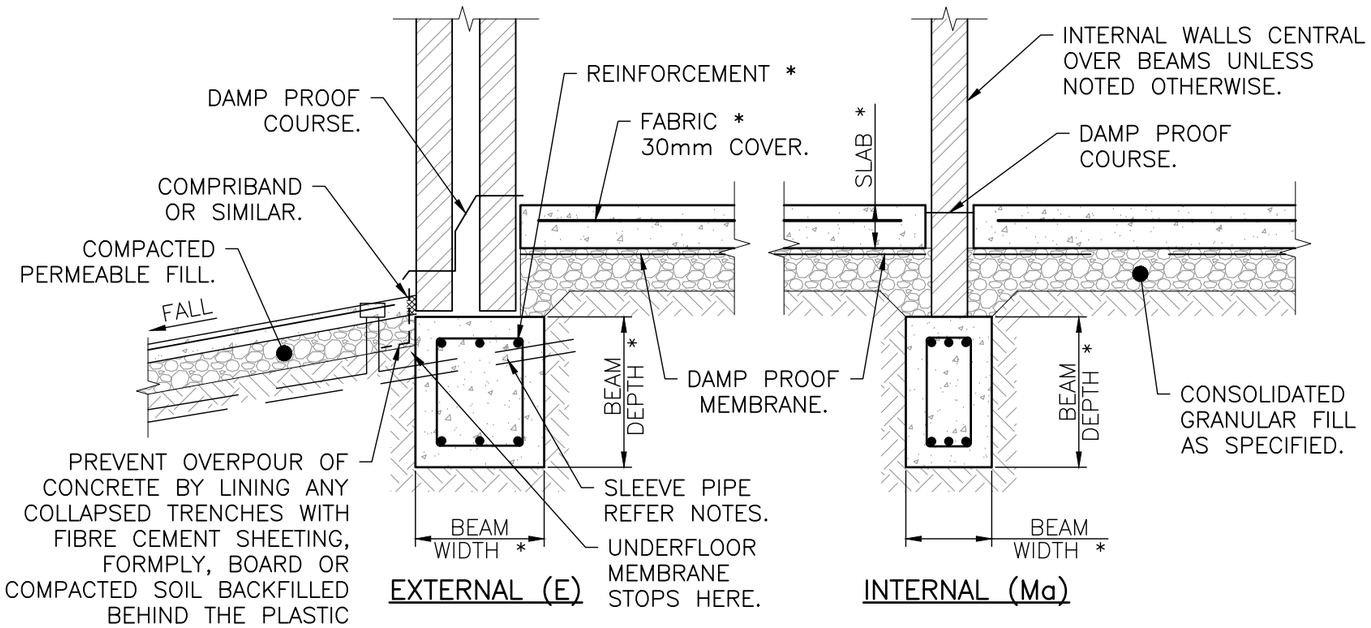
FOOTING ELEVATION

NOTES

1. MAIN BARS TO BE CONTINUOUS WHERE POSSIBLE SPLICES WHEN NECESSARY AS SPECIFIED *.
2. * – REFER TO CONSTRUCTION REPORT.
3. ALL DIMENSIONS ARE IN MILLIMETRES.

STRIP FOOTING GENERAL DETAILS

SCALE N.T.S



EXTERNAL (E)

INTERNAL (Ma)

NOTES

1. CLEAR CONCRETE COVER TO REINFORCEMENT TO BE 50mm FOR BEAMS AND 30mm FOR SLABS.
2. * – REFER TO CONSTRUCTION REPORT.
3. ALL PIPES PENETRATING FOOTING BEAMS SHALL BE LAGGED WITH CLOSED CELL POLYETHYLENE LAGGING (40mm FOR CLASS E & H SITES AND 20mm FOR CLASS M & S SITES).
4. TOP OF PIPE RISES TO 15mm ABOVE PAVING OR 75mm ABOVE GROUND IF PAVING DELETED.

STRIP FOOTING TYPICAL BEAM CROSS SECTIONS

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